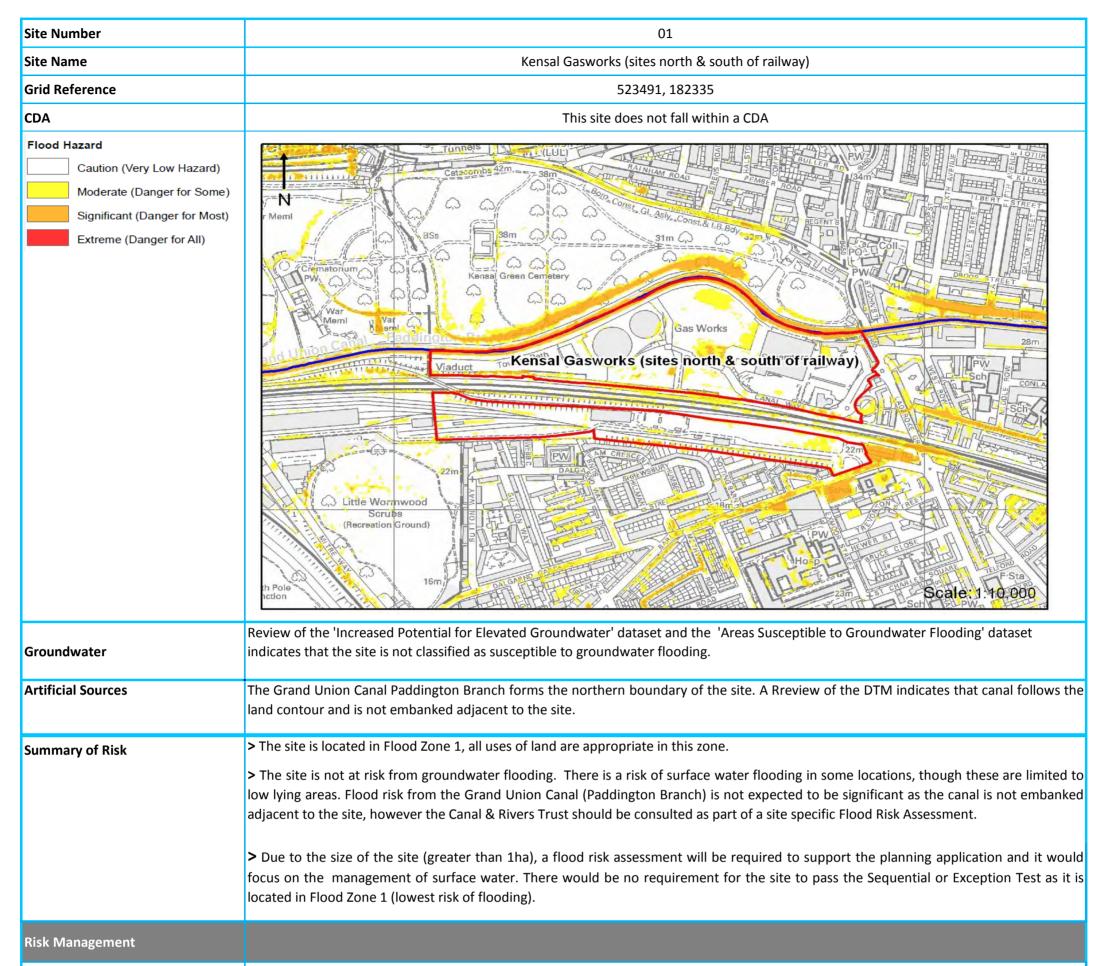


Topography	> Ground levels on the site range between 25mAOD and 30mAOD
Risk Assessment	
Flood Zones	Proportion of the site located in:-
	Flood Zone 1 = 18ha (low risk of flooding)
	Flood Zone 2 = 0ha
	Flood Zone 3a = 0ha
	Flood Zone 3b = 0ha
Surface Water (Pluvial)	Surface water modelling undertaken for the Royal Borough of Kensington and Chelsea Surface Water Management Plan predicts some areas of ponding on the site during the 1 in 100 year rainfall event with an allowance for climate change. These areas of ponding are associated with a moderate (danger for some) and significant (danger for most) hazard rating. Refer to the figures below for the 1 in 100 year rainfall event with an allowance for climate change.
Flood Depth (m)	
< 0.1m	
0.1m to 0.25m	
0.25m to 0.5m	
0.5m to 1.0m	
1.0m to 1.5m	Crematorium College Constant
> 1.5m	
	Gas Works
	Vieture 10 Kensal Gasworks (sites north & south of railway)
	Vieduct Torrensal Gasworks (sites north a south of failway)
	h Pole



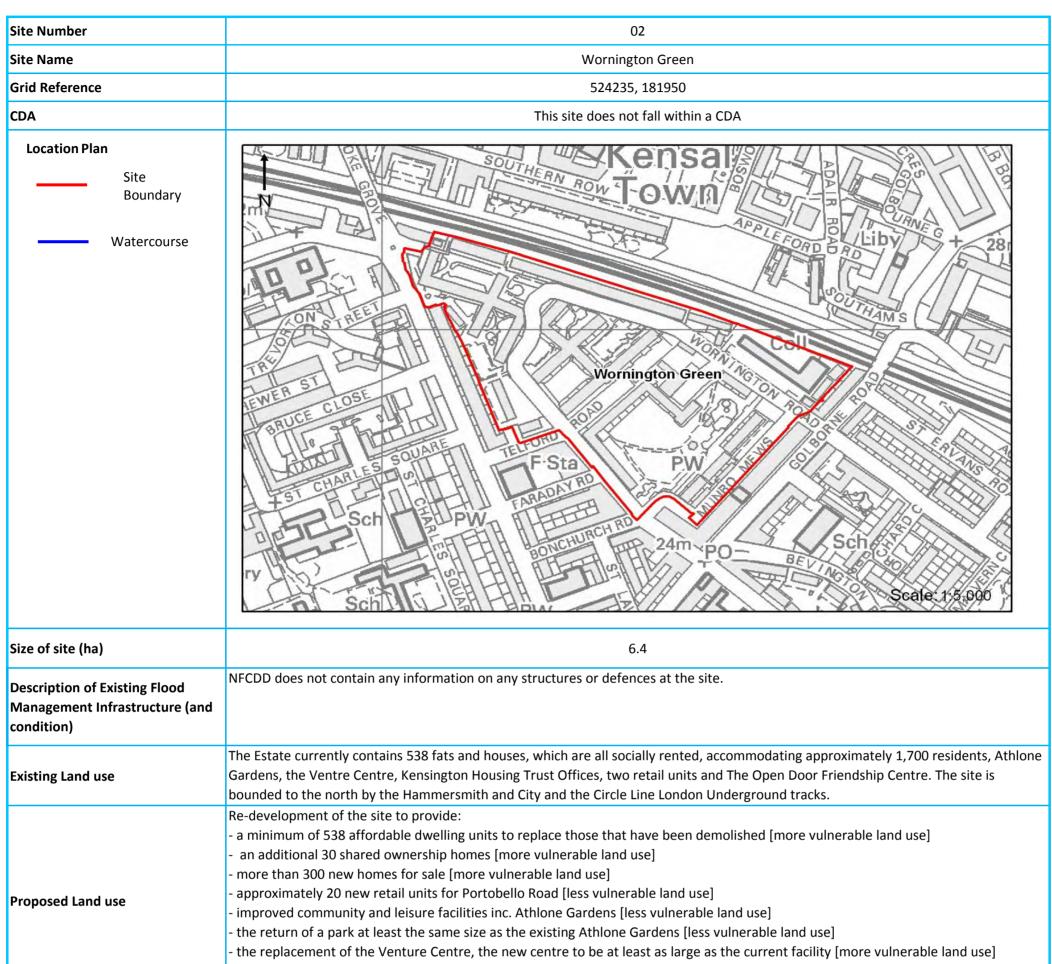




Flood risk management recommendations	> The design and layout of the proposed development should seek as much as possible to avoid impacting overland flow routes within the site, which may increase flood risk elsewhere.
	> A review of the DTM indicates that the Grand Union Canal (Paddington Branch) is not embanked adjacent to the site. However, it is recommended the Canal & River Trust are consulted about any future development as part of a site specific Flood Risk Assessment.
	> Ground floor levels should be above surrounding ground levels to prevent ingress of surface water runoff. This should be agreed with the EA at the earliest opportunity.
	> There is a large grassed area between to the Gasworks and the Sainsbury supermarket. Any development in this area is likely to result in an increase in surface water runoff, however this can be appropriately managed through the development of a SUDS treatment train for the site.
SUDS Options appraisal	> The site is underlain by London Clay and typically does not have a high level of permeability. Therefore there maybe limited opportunity to utilise infiltration based SUDS techniques at the site.
	> All SUDS measures are suitable depending on the final layout and results of permeability testing of the insitu soils. It is recommended that infiltration testing is undertaken to determine the suitability of infiltration devices within the site.
Reasonable prospect of compl with the Exception Test?	iance > The site is fully located in Flood Zone 1 and therefore there is no need to apply the Exception Test.



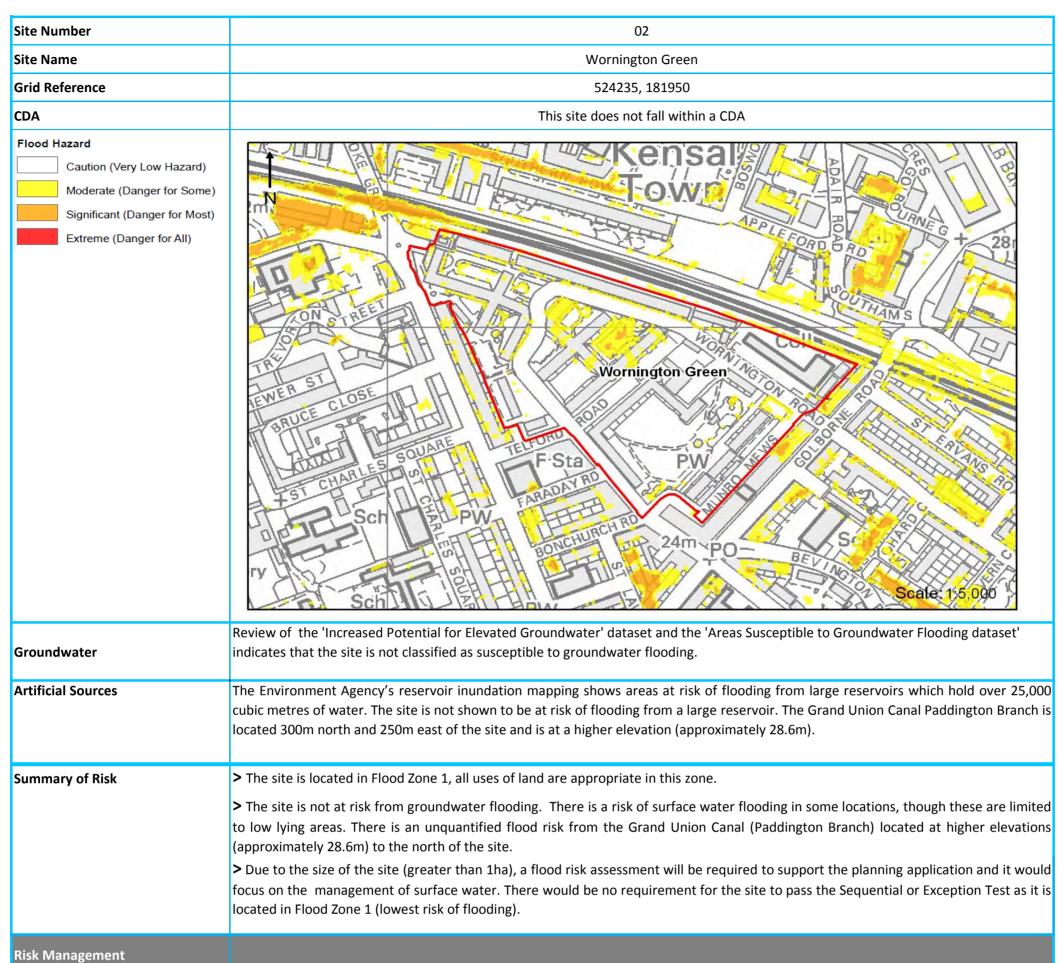




	- the replacement of the lock-ups on Munro Mews [less vulnerable land use]
Topography	> Ground levels on the site range between 23mAOD and 26mAOD
Risk Assessment	
Flood Zones	Proportion of the site located in:- Flood Zone 1 = 6.4ha (low risk of flooding) Flood Zone 2 = 0ha Flood Zone 3a = 0ha Flood Zone 3b = 0ha
Surface Water (Pluvial)	Surface water modelling undertaken for the Royal Borough of Kensington and Chelsea Surface Water Management Plan predicts some areas of ponding on the site during the 1 in 100 year rainfall event with an allowance for climate change. These areas of ponding are associated with a moderate (danger for some) and two small areas of significant (danger for most) hazard rating. Refer to the figures below for the 1 in 100 year rainfall event with an allowance for climate change.
Flood Depth (m)         < 0.1m	Your School       Your School





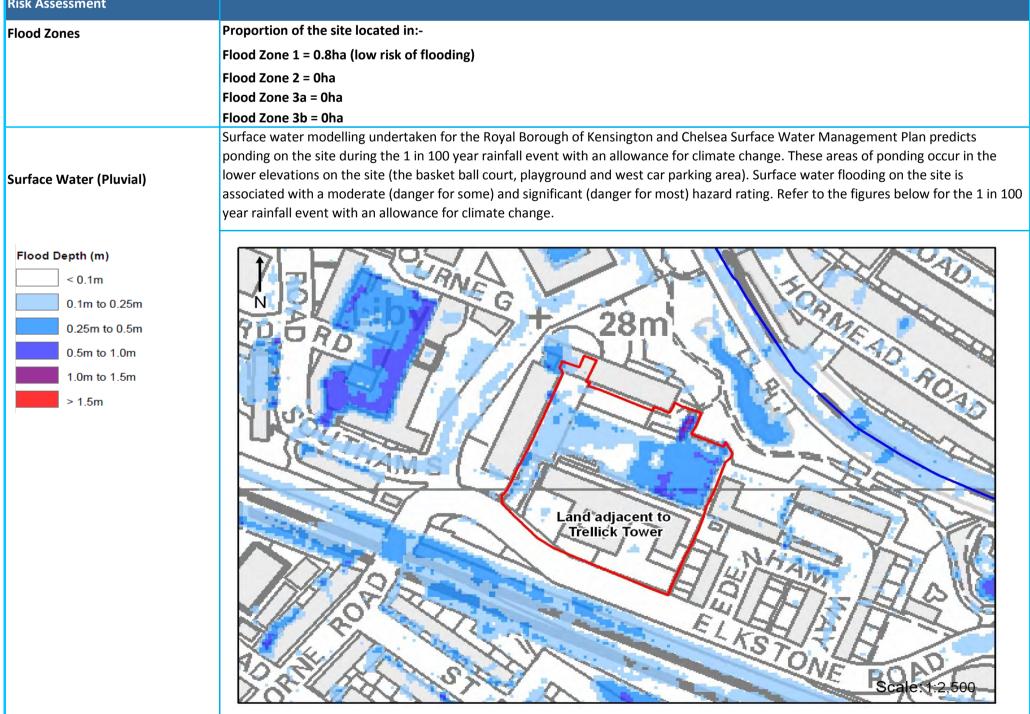


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Flood risk management recommendations	> The design and layout of the re-development should seek as much as possible to avoid impacting overland flow routes within the site, which may increase flood risk elsewhere.
	> There is an unquantified flood risk from the Grand Union Canal (Paddington Branch) located at a higher elevation (approximately 28.6m) to the north of the site. It is recommended that the Canal & Rivers Trust should be consulted as part of a site specific Flood Risk Assessment.
	> Ground floor levels should be above surrounding ground levels to prevent ingress of surface water runoff. This should be agreed with the EA at the earliest opportunity.
	> Re-development of in this area is unlikely to result in an increase in surface water runoff if the current area of green space is retained. Improvements to the current situation may be achieved and surface water runoff can be appropriately managed through the development of a SUDS treatment train for the site.
SUDS Options appraisal	> The site is underlain by London Clay and typically does not have a high level of permeability. Therefore there maybe limited opportunity to utilise infiltration based SUDS techniques at the site.
	> All SUDS measures are suitable depending on the final layout and results of permeability testing of the insitu soils. It is recommended that infiltration testing is undertaken to determine the suitability of infiltration devices within the site.
Reasonable prospect of compliance with the Exception Test?	> The site is fully located in Flood Zone 1 and therefore there is no need to apply the Exception Test.



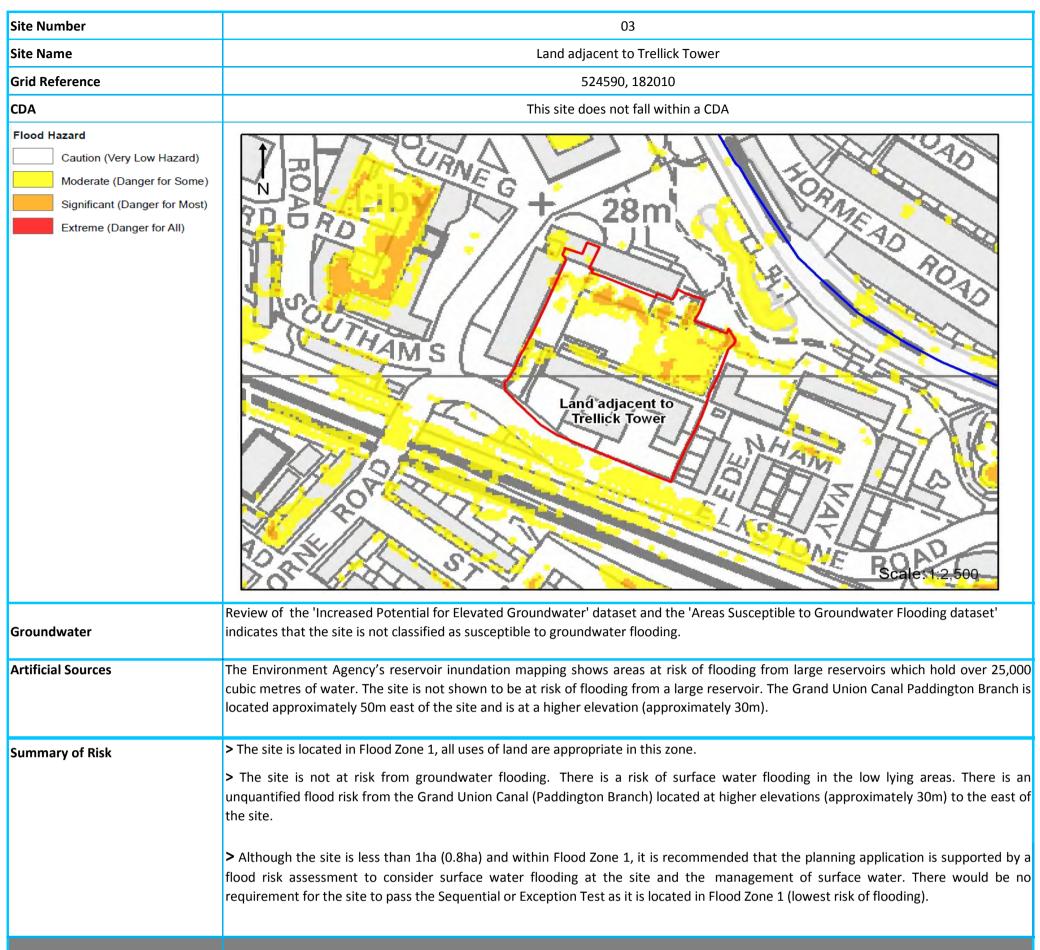


Site Number	03
Site Name	Land adjacent to Trellick Tower
Grid Reference	524590, 182010
CDA	This site does not fall within a CDA
Location Plan Site Boundary Watercourse	Land'adjacent to Trellick Tower
Size of site (ha)	0.8
Description of Existing Flood Management Infrastructure (and condition)	NFCDD does not contain any information on any structures or defences at the site.
Existing Land use	The site encompasses the former Edenham Care Home and land adjoining Trellick Tower, which is located in the north-east of the Borough and is situated in Golborne Ward. The site largely comprises of car parking and recreational area and gardens.
Proposed Land use	The development of the site to include: - new residential units [more vulnerable land use] - improvements to social and community facilities and housing
Topography	> Ground levels on the site vary. The gardens around the base of Trellick Tower are at elevations of approximately 29m. The basket ball court, playground and west car parking area which are at lower elevations compared to the rest of the site (approximately 23m to 23.5m). The car park area in the south of the site is approximately 25.5m
Risk Assessment	





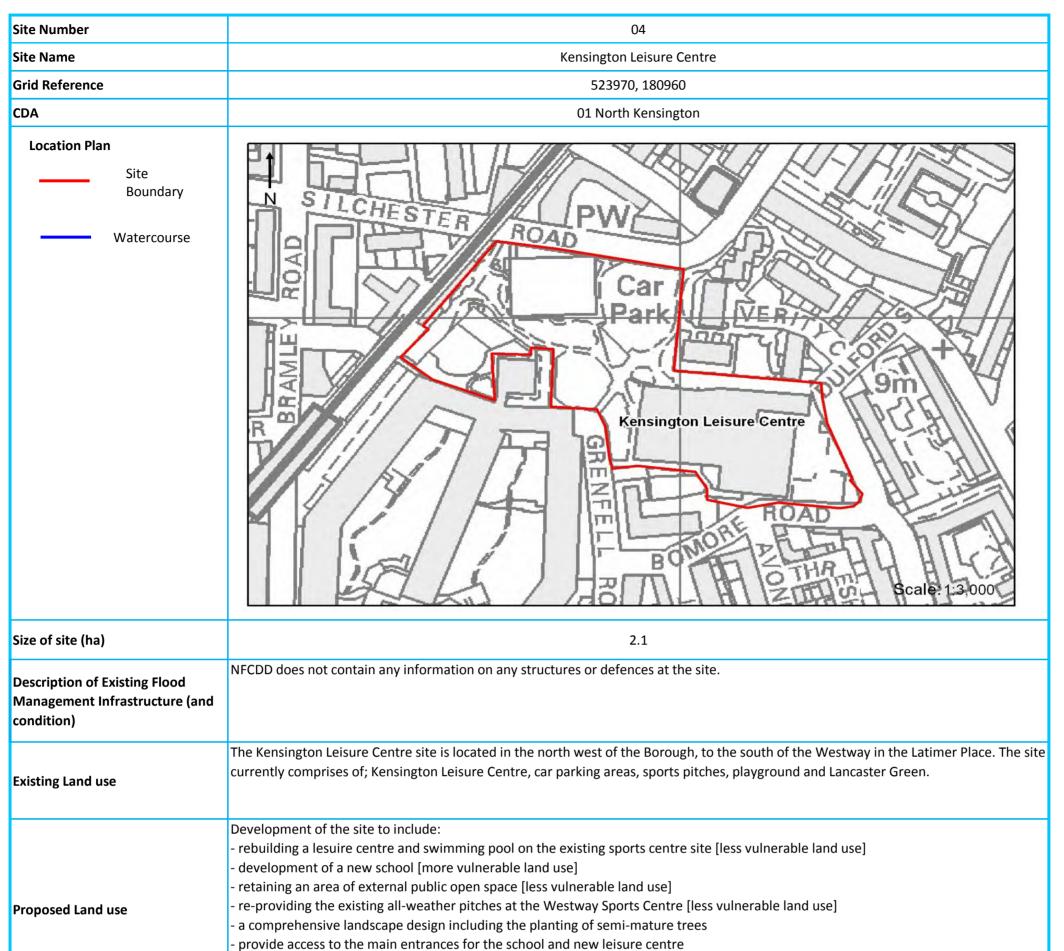




> The design and layout of the proposed development should seek as much as possible to avoid impacting overland flow routes within the site, which may increase flood risk elsewhere.
> There is an unquantified flood risk from the Grand Union Canal (Paddington Branch) located at higher elevations (approximately 30m) to the east of the site. It is recommended that the Canal & Rivers Trust should be consulted as part of a site specific Flood Risk Assessment.
> Ground floor levels should be above surrounding ground levels to prevent ingress of surface water runoff. This should be agreed with the EA at the earliest opportunity.
> Development of this site is likely to result in an increase in surface water runoff, however this can be appropriately managed through the development of a SUDS treatment train for the site.
> The site is underlain by London Clay and typically does not have a high level of permeability. Therefore there maybe limited opportunity to utilise infiltration based SUDS techniques at the site.
> All SUDS measures are suitable depending on the final layout and results of permeability testing of the insitu soils. It is recommended that infiltration testing is undertaken to determine the suitability of infiltration devices within the site.
> The site is fully located in Flood Zone 1 and therefore there is no need to apply the Exception Test.



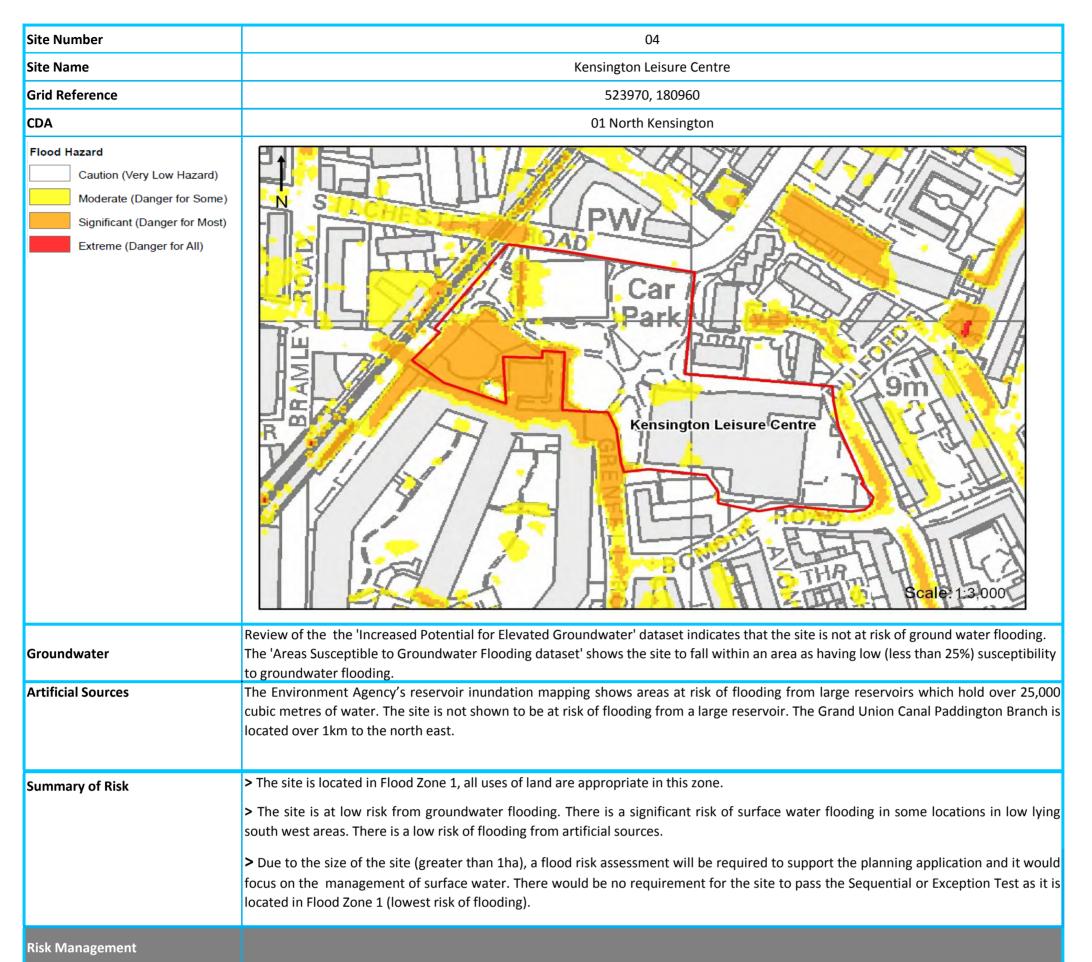




	<ul> <li>provide access to the main entrances for the school and new leisure centre</li> <li>new housing, so long as the housing does not compromise any of the other objectives for the site [more vulnerable land use]</li> </ul>
Topography	> Ground levels on the site range between 6mAOD (in the south west of the site) and 9mAOD
Risk Assessment	
Flood Zones	Proportion of the site located in:- Flood Zone 1 = 2.1ha (low risk of flooding) Flood Zone 2 = 0ha Flood Zone 3a = 0ha Flood Zone 3b = 0ha
Surface Water (Pluvial)	Surface water modelling undertaken for the Royal Borough of Kensington and Chelsea Surface Water Management Plan predicts ponding on the site during the 1 in 100 year rainfall event with an allowance for climate change. The deepest areas of ponding occur in the lower elevations in the south west area. Surface water flooding on the site is associated with a moderate (danger for some) and significant (danger for most) hazard rating in the south west area. Refer to the figures below for the 1 in 100 year rainfall event with an allowance for climate change.
Flood Depth (m)         < 0.1m	Resington Leisure Centre Kensington Leisure Centre Scal et 1:3:000



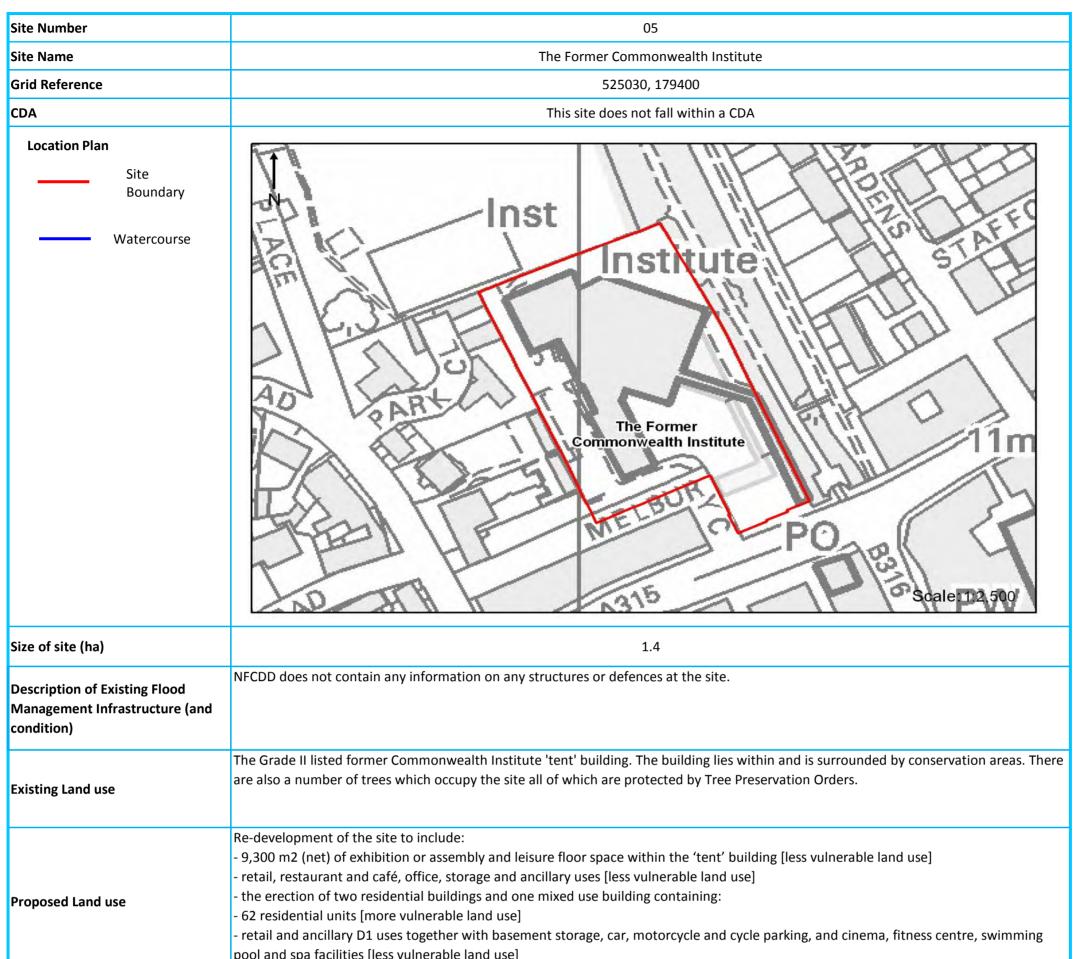




Flood risk management recommendations	> The design and layout of the proposed development should seek as much as possible to avoid impacting overland flow routes within the site, which may increase flood risk elsewhere.
	> Ground floor levels should be above surrounding ground levels to prevent ingress of surface water runoff. This should be agreed with the EA at the earliest opportunity.
	> An increase in impermeable surfaces from development of this site is likely to result in an increase in surface water runoff, however this can be appropriately managed through the development of a SUDS treatment train for the site.
SUDS Options appraisal	> The site is underlain by London Clay bedrock, which typically has a low level of permeability, and superficial deposits from the Langley Silt Member (which varies from clay to silt) in the south and west of the site. Therefore there maybe limited opportunity to utilise infiltration based SUDS techniques at the site.
	> All SUDS measures are suitable depending on the final layout and results of permeability testing of the insitu soils. It is recommended that infiltration testing is undertaken to determine the suitability of infiltration devices within the site.
Reasonable prospect of compliance with the Exception Test?	> The site is fully located in Flood Zone 1 and therefore there is no need to apply the Exception Test.



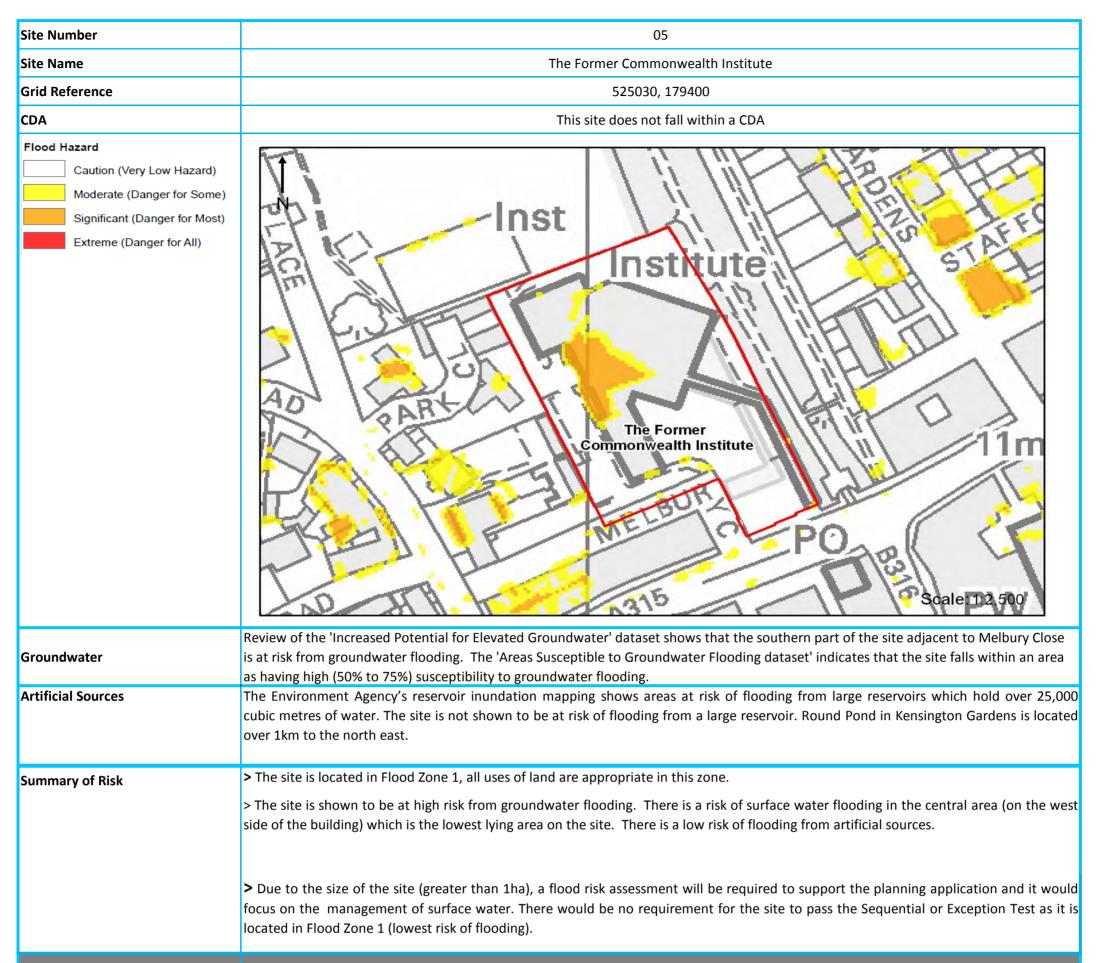




	pool and spa facilities [less vulnerable land use]
Topography	> Ground levels on the site range between 9mAOD and 15mAOD
Risk Assessment	
Flood Zones	Proportion of the site located in:- Flood Zone 1 = 1.4ha (low risk of flooding) Flood Zone 2 = 0ha Flood Zone 3a = 0ha Flood Zone 3b = 0ha Surface water modelling undertaken for the Royal Borough of Kensington and Chelsea Surface Water Management Plan predicts deep
Surface Water (Pluvial)	ponding in the central area of the site (on the west side of the building) during the 1 in 100 year rainfall event with an allowance for climate change. This area of ponding is associated with a significant (danger for most) hazard rating. Refer to the figures below for the 1 in 100 year rainfall event with an allowance for climate change.
Flood Depth (m)          0.1m         0.1m to 0.25m         0.25m to 0.5m         0.5m to 1.0m         1.0m to 1.5m         > 1.5m	Inst Inst Unstrute The Former Commonwealth Institute



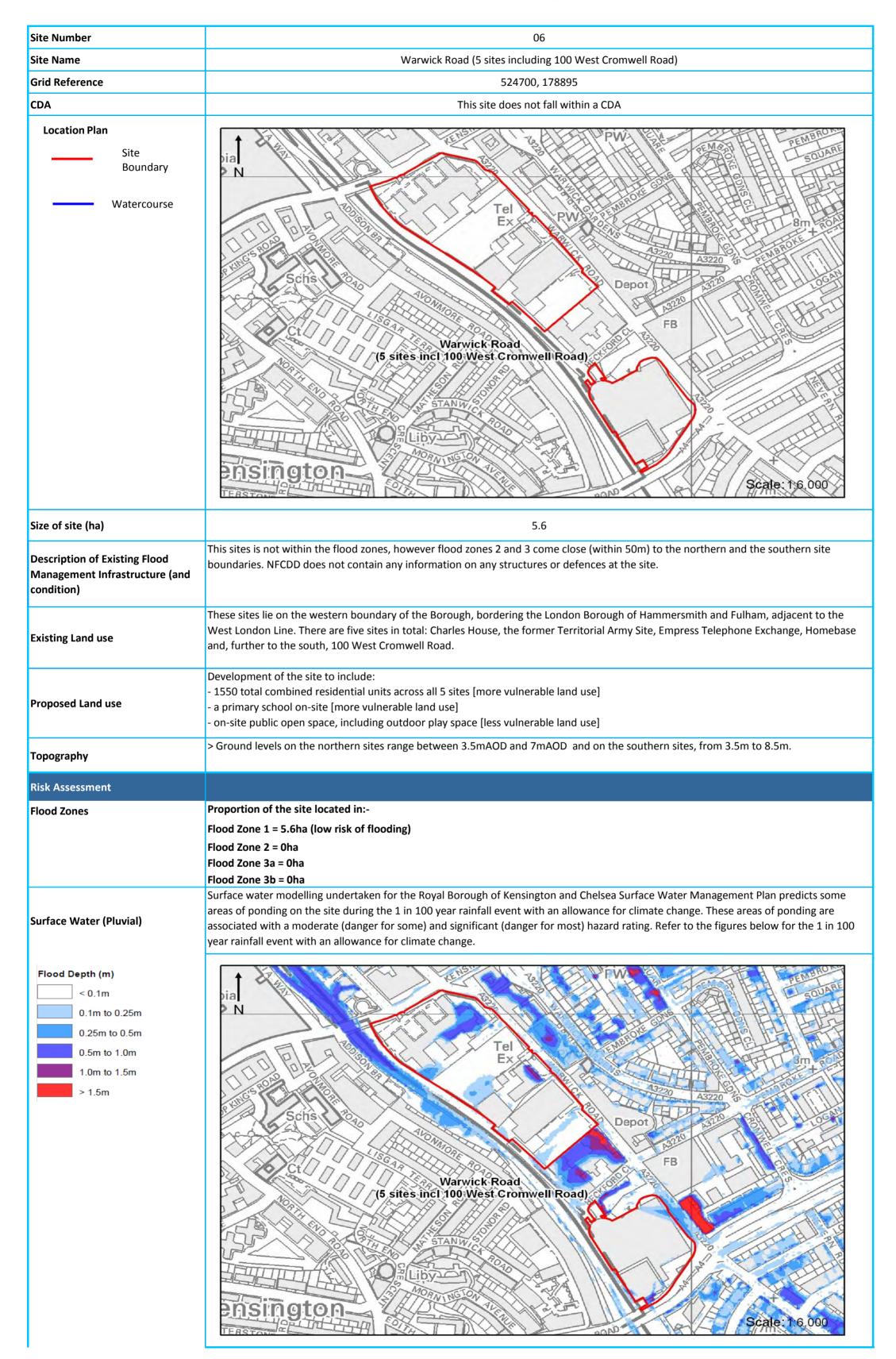




Risk Management	
Flood risk management recommendations	> The design and layout of the proposed development should seek as much as possible to avoid impacting overland flow routes within the site, which may increase flood risk elsewhere.
	> The surface water flood risk to the site should be investigated as part of the site specific flood risk assessment. Re-grading/ landscaping of the grounds and the incorporation of SUDs should be considered to prevent ponding directly next to the building.
	> The existing ground floor levels of the building should be assessed and (if required) options to prevent the ingress of groundwater and surface water runoff into the building should be considered.
	> The building is surrounded by grassed areas providing space and opportunity to incorporate SUDs. Re-development of in this area is unlikely to result in an increase in surface water runoff if the current area of green space is retained. Improvements to the current situation may be achieved and surface water runoff can be appropriately managed through the development of a SUDS treatment train for the site.
SUDS Options appraisal	> The site is underlain by London Clay bedrock, which typically has a low level of permeability, and superficial deposits from the Taplow Gravel Formation (sands and gravels). Therefore there maybe limited opportunity to utilise infiltration based SUDS techniques at the site.
	> All SUDS measures are suitable depending on the final layout and results of permeability testing of the insitu soils. It is recommended that infiltration testing is undertaken to determine the suitability of infiltration devices within the site.
Reasonable prospect of compli with the Exception Test?	ance > The site is fully located in Flood Zone 1 and therefore there is no need to apply the Exception Test.

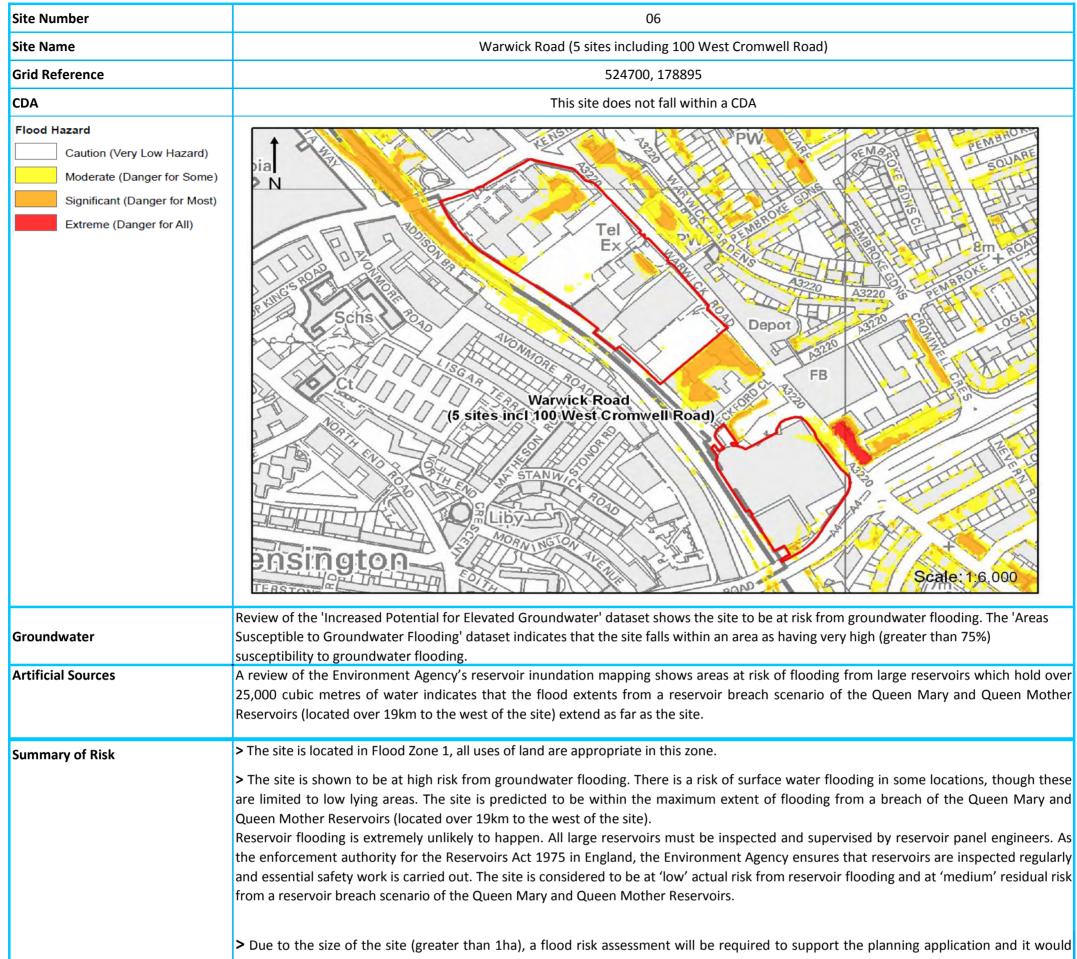








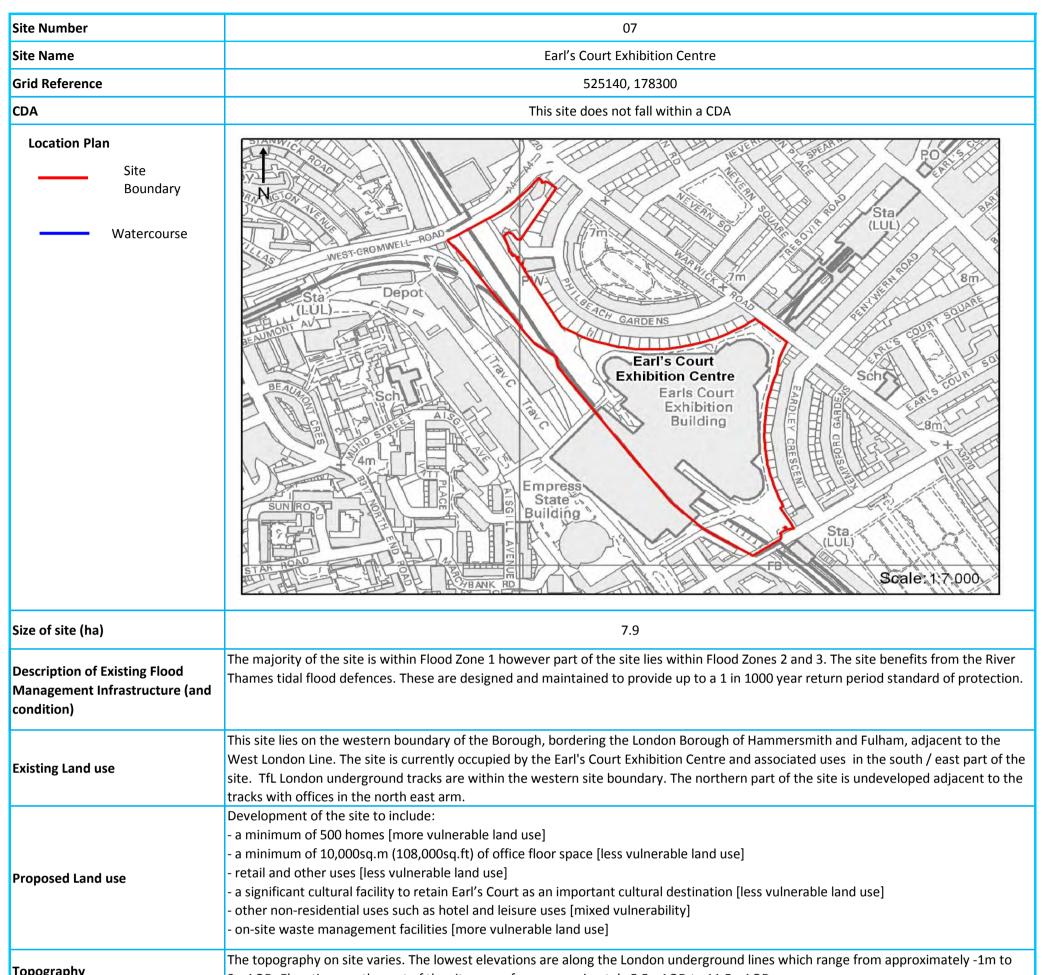




	focus on the management of surface water. There would be no requirement for the site to pass the Sequential or Exception Test as it is located in Flood Zone 1 (lowest risk of flooding).
Risk Management	
Flood risk management recommendations	> The design and layout of the proposed development should seek as much as possible to avoid impacting overland flow routes within the site, which may increase flood risk elsewhere.
	> Thames Water Ltd own the Queen Mary and Queen Mother Reservoirs. The site developer should contact the local authorities who are responsible for developing emergency plans in the event of a reservoir breach.
	> Ground floor levels should be above surrounding ground levels to prevent ingress of surface water runoff and groundwater flooding. This should be agreed with the EA at the earliest opportunity.
	> An increase in impermeable surfaces from development of this site is likely to result in an increase in surface water runoff, however this can be appropriately managed through the development of a SUDS treatment train for the site.
SUDS Options appraisal	> The site is underlain by London Clay bedrock, which typically has a low level of permeability, and superficial deposits from the Kempton Park Gravel Formation (sand and gravels). Therefore there maybe limited opportunity to utilise infiltration based SUDS techniques at the site.
	> All SUDS measures are suitable depending on the final layout and results of permeability testing of the insitu soils. It is recommended that infiltration testing is undertaken to determine the suitability of infiltration devices within the site.
Reasonable prospect of complian with the Exception Test?	nce > The site is fully located in Flood Zone 1 and therefore there is no need to apply the Exception Test.



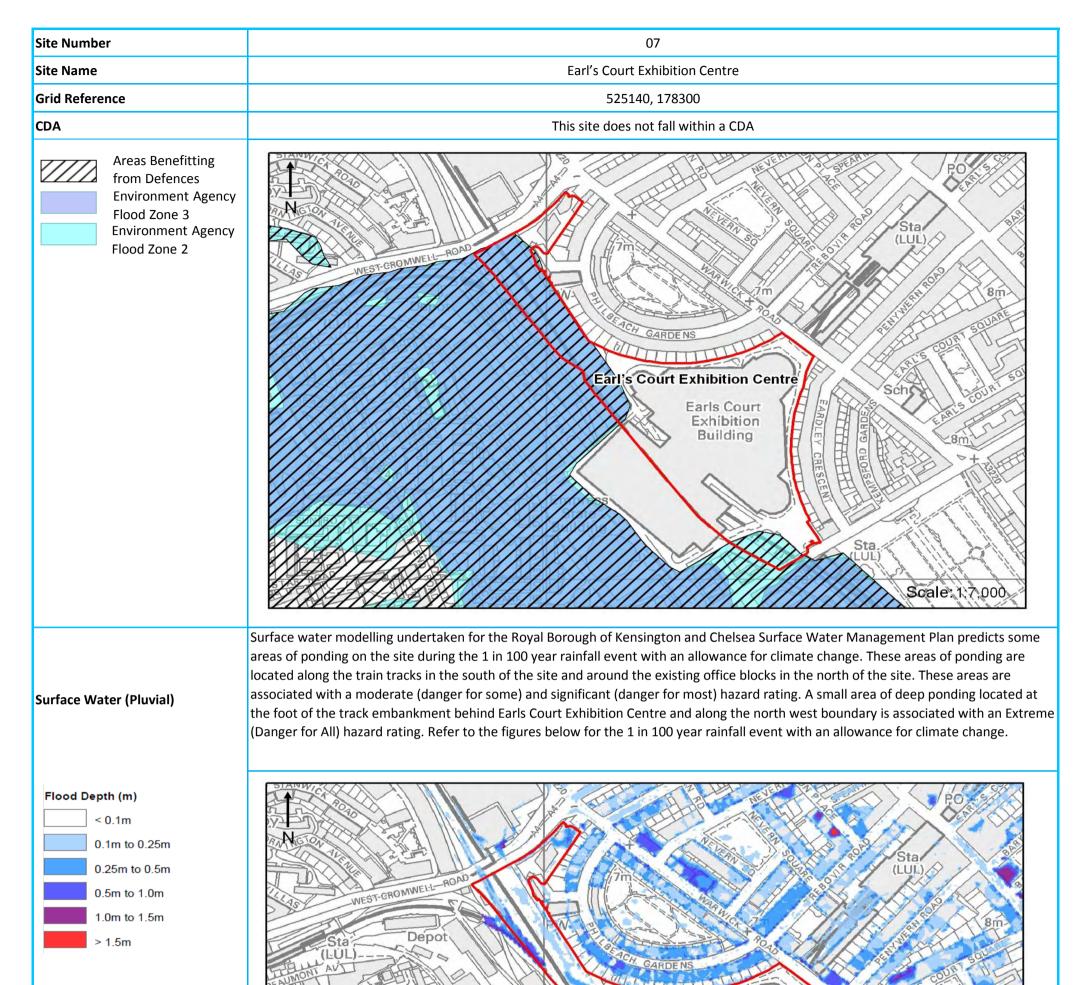




Topography	3mAOD. Elevations on the rest of the site range from approximately 5.5mAOD to 11.5mAOD.
Risk Assessme	
Flood Zones	Proportion of the site located in:-
	Flood Zone 1 = 6.1ha (low risk of flooding)
	Flood Zone 2 = 1.8ha
	Flood Zone 3a = 1.5ha
	Flood Zone 3b = unknown
	(3) The site is predominantly located in Flood Zone 1, the north / west area is within Flood Zone 3 and a small area in the south of the
	site is within Flood Zone 2 (refer to the figure below). The site benefits from the River Thames tidal flood defences. These are
	designed and maintained to provide up to a 1 in 1000 year return period standard of protection.



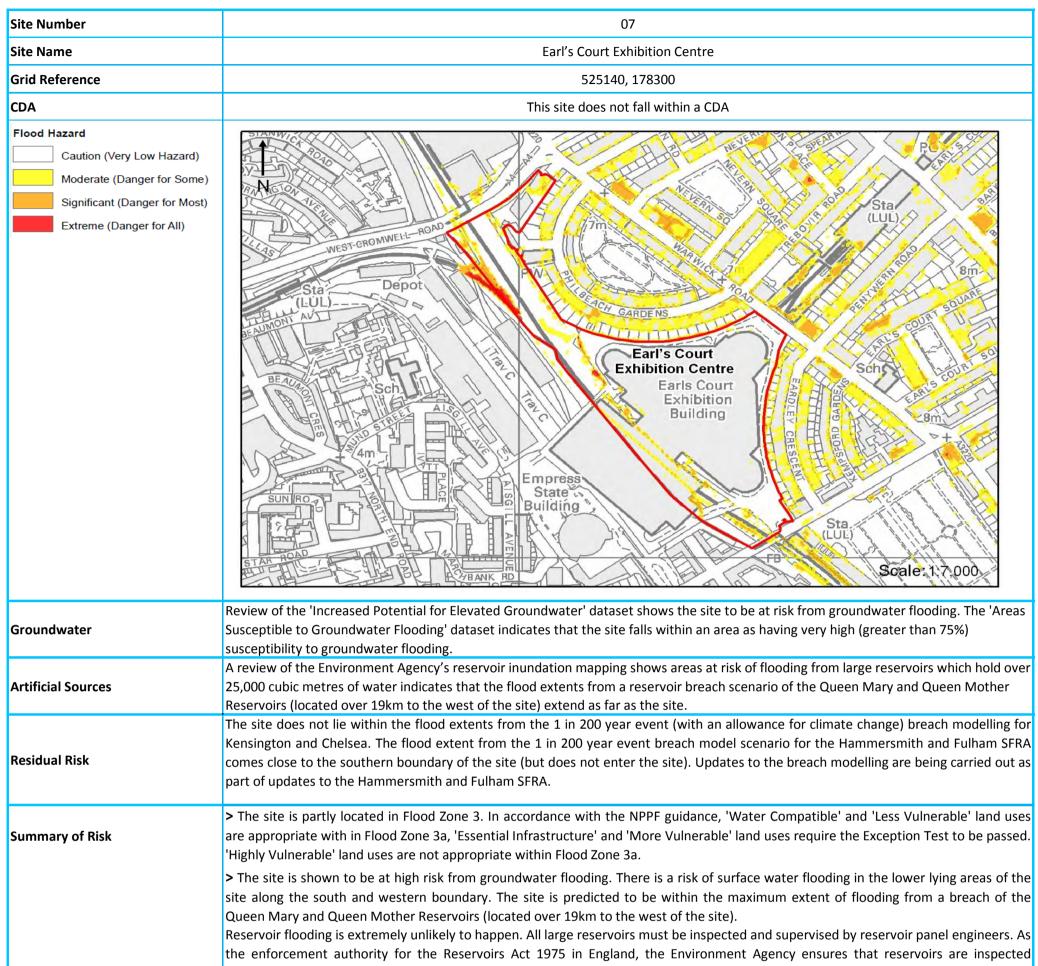












	regularly and essential safety work is carried out. The site is considered to be at 'low' actual risk from reservoir flooding and at 'medium' residual risk from a reservoir breach scenario of the Queen Mary and Queen Mother Reservoirs.
	> The flood extent from the 1 in 200 year event breach model scenario for the Hammersmith and Fulham SFRA comes close to the southern boundary of the site (but does not enter the site). The site is currently considered to be at 'low' actual risk from a breach of the Thames flood defences, however updates to the breach modelling for the Hammersmith and Fulham SFRA should be consulted as part of a site specific flood risk assessment.
Risk Management	
Flood risk management recommendations	A flood risk assessment will be required to support the planning application for this site. The Sequential Test will need to be applied for all types of development within the development site, however, if there is More Vulnerable development proposed within the 1% AEP flood extent the Exception Test must also be applied. Substitution of More Vulnerable development for any Less Vulnerable development within the 1% AEP development within the 1% AEP flood extent the 1% AEP flood extent is recommended wherever possible. Replacing More Vulnerable uses on the ground floor with Less Vulnerable uses on the ground floor may also be appropriate.
	> The design and layout of the proposed development should seek as much as possible to avoid impacting overland flow routes within the site, which may increase flood risk elsewhere.
	> Thames Water Ltd own the Queen Mary and Queen Mother Reservoirs. The site developer should contact the local authorities who are responsible for developing emergency plans in the event of a reservoir breach.
	> Updates to the breach modelling for the Hammersmith and Fulham SFRA should be consulted as part of a site specific flood risk assessment.
	> Ground floor levels should be set above surrounding ground levels to prevent ingress of groundwater and surface water. This should be agreed with the EA at the earliest opportunity.
	> An increase in impermeable surfaces from development of this site (in particular the green areas adjacent to the train tracks) is likely to result in an increase in surface water runoff, however this can be appropriately managed through the development of a SUDS treatment train for the site.
SUDS Options appraisal	The site is underlain by London Clay bedrock, which typically has a low level of permeability, and superficial deposits from the Kempton Park Gravel Formation (sand and gravels). Therefore there maybe limited opportunity to utilise infiltration based SUDS techniques at the site.





Site Number	07	
Site Name	Earl's Court Exhibition Centre	
Grid Reference	525140, 178300	
CDA	This site does not fall within a CDA	
	> All SUDS measures are suitable depending on the final layout and results of permeability testing of the insitu soils. It is recommended that infiltration testing is undertaken to determine the suitability of infiltration devices within the site.	
Reasonable prospect of compliance with the Exception Test?	> The site is partly located in Flood Zone 3. If either 'Essential Infrastructure' and 'More Vulnerable' land uses are located within Flo Zone 3a, the Exception Test must be passed. There is a reasonable prospect of the site complying with the Exception Test: The site h been allocated The site benefits from the River Thames tidal flood defences designed and maintained to provide up to a 1 in 1000 ye return period standard of protection. Breach modelling shows that the site is not predicted to be at residual risk of flooding from a breach of the tidal defences. The site specific flood risk assessment must demonstrate that development will be safe for its lifetime.	